GEOTECHNICAL INVESTIGATION REPORT FOR STORM WATER DRAINAGE SCHEME OF MUZAFFARPUR CITY

CLIENT DARASHAW AND COMPANY PRIVATE LIMITED, HYDERABAD



SIRI GEOSOLUTIONS PVT LTD

An ISO 9001-2008 Company
OFFICE 419, 4th Floor, kubera Towers, Narayanaguda, HYDERABAD -500 029
PHONE: +91-9493372677, www.sirigeosolutions.com
Email. info@sirigeosolutions.ccom, consultancy.geo@gmail.com

CONTENTS

1	INTRODUCTION	. 1
2	SCOPE OF WORK	. 1
3	FIELD WORK	. 1
4	LABORATORY TESTS	. 2
5	SUBSOIL CONDITION	. 3
6	DESIGN METHODOLOGY	. 3
7	FOUNDATION RECOMMENDATIONS	. 4

Annexure

Annexure - I : Borehole Location Plan

Annexure - II : Field Bore Logs

Annexure - III : Laboratory Test Results

Annexure - IV : SBC Calculations

1 INTRODUCTION

M/s Darashaw and Company Private Limited, Hyderabad has entrusted the work to conducting geotechnical investigations for the proposed detailed project report for Storm Water Drainage Scheme in Muzaffarpur City in Bihar to M/s Siri GeoSolutions Pvt Ltd., Hyderabad.

Objective of the geotechnical investigation is to ascertain sub-soil parameters for appropriate design of the foundation system.

2 SCOPE OF WORK

The scope of work in Muzaffarpur Municipal Corporation is to make specified number of standard geotechnical auger boreholes and trial pits at specified location as per the specifications and direction given by the client.

The detailed scope of work includes the following

- Making one standard geotechnical auger boreholes and two trial pits at specified locations shown by the client.
- ♣ Auger drilling through soil strata upto ten meters or refusal as pre client directions.
- ♣ Conducting standard penetration test at 1.5m depth intervals.
- Collecting disturbed and undisturbed soil samples.
- Conducting relevant field and laboratory tests.
- Submitting comprehensive geotechnical report which covering all field works, laboratory test results and recommendations for the foundation system

3 FIELD WORK

Field investigations were carried out during the month of October 2014. Brief methodologies of boring and other field works are given below.

3.1 Auger Boring

Exploratory boreholes of 100mm diameter were drilled in soil strata by screw type auger. The description of sub soil stratum with SPT 'N' values at regular intervals were recorded in the bore logs and presented in Annexure -II.

Details of boreholes, depth of exploration levels are presented in the table below.

SI No.	Location	No. of Boreholes	BH No.	Depth of Explored (m)
1	Ref Annexure-1	1	BH-1	10.00

3.2 Sampling

Both disturbed and undisturbed representative soil samples were collected wherever possible during boring and subsequently sent to laboratory for visual examination and laboratory tests.

3.3 Standard Penetration Test

In borehole, standard penetration tests were conducted at 1.5m interval or change in strata as per Indian Standard, IS:2131-1981and split spoon sampler used conformed to the Indian Standard, IS:9640-1980. These tests were performed by driving the sampler with 63.5kg hammer falling from a height of 75cm. Number of blows required to effect each 15cm penetration is recorded .The first 15cm penetration is considered to be the seating drive. The sum of the total blows required for a second and third 15cm (total 30cm) penetration is termed the penetration resistance SPT' N' value and the same is reported.

When the number of blows exceeded 50 to penetrate the first or second 15cm length of the sampler, the SPT N is regarded as more than 100 as described in IS 2131 - 1981. The test is terminated in such case and a record of penetration of the sampler under 50 blows is made. SPT refusal is recorded when there is no penetration of the sampler at any stage and also when a rebound of the sounding system is recorded. Field SPT'N' values are given in bore logs in Annexure- II.

3.4 Trial Pit

Test pits of 1.0m x 1.0m x 2.50m deep were excavated at specified locations as per direction of client specifications. The purposes of the test pits are visually examine the sub soil stratum. The locations of test pits are tabulated below.

SI. No	Trial Pit	Location	Depth of pit (m)
1	TP-1	Ref Annexure-1	2.50
2	TP-2	Ref Annexure-1	2.50

4 LABORATORY TESTS

Laboratory tests were carried out on soil samples as per the relevant Indian Specifications. Details of various tests conducted are presented below.

Geotechnical Investigation Report for Storm Water Drainage Scheme of Muzaffarpur City.

A) Tests on soil samples:

The following tests have been performed for soil samples:

Moisture contents;

Grain size analysis tests

Atterberg's limits-liquid limit and plastic limit;

Triaxial Compression tests;

On sandy samples grain size distribution analysis test were conducted using both dry and wet sieve analysis techniques. These tests are completed primarily to classify and to find out the gradation characteristics.

In general on clayey samples Atterberg's Limit tests are conducted to derive the engineering characteristic.

The laboratory test results of soil sample are given in Annexure -III.

5 SUBSOIL CONDITION

One auger borehole and two trial pits were drilled at proposed project site. The borehole and trial pits were terminated as per the direction of Engineer in-charge the depth is indicated at bottom of bore log. Details of the SPT"N" values conducted at various depths are presented in individual bore logs with IS classification in Annexure-II.

From the field observations and laboratory test results the brief descriptions of the various stratums are presented below.

Stratum-I (clayey silty)

The top stratum is comprised to grayish yellow clayey silt till the borehole termination. The SPT 'N' values are vary from 2 to 8. As per grain size and Atterberg limit values this soil can be classified CL as per IS classification. The similar clayey silt stratum was observed in trail pits.

6 DESIGN METHODOLOGY

The safe bearing capacity is arrived based on the shear failure criteria and settlement criteria. The estimated safe bearing capacity is calculated as per IS: 6403-1981 code and IS: 8009 (Part-1)-1976 with available soil profile.

A properly designed foundation has to satisfy two limit states.

1) Limit state of collapse (ie., Shear strength)

2) Limit state of serviceability (ie., Settlement)

Shear Criteria

The first criterion is depending on shear strength. The calculations are based on bearing capacity equation as recommended by IS:6403 (with factor of Safety 2.5) which takes care of L/B ratio (shape), foundation depth etc., along with other parameters.

The intensity of loading at the base of the foundation, which would cause shear failure of the soil support, is termed as ultimate bearing capacity (UBC).

The following equation is used to calculate ultimate bearing capacity,

$$UBC = C*Nc*Sc*dc + q*Nq*Sq*dq + 0.5*B*\gamma*N\gamma*S\gamma*d\gamma*W'$$

Safe bearing capacity (SBC) is the maximum intensity of loading that the foundation will safely carry without the risk of shear failure of soil irrespective of any settlement that may occur. Safe bearing capacity can be obtained by dividing the UBC with suitable factor of safety (2.5).

$$SBC = UBC/2.5$$

Settlement Criteria:

The intensity of loading that will cause a permissible settlement or specified settlement of the structure is termed as allowable bearing pressure.

The elastic settlement of granular soil and consolidated settlement of cohesive soils are calculated using SPT 'N' values and laboratory test results. Settlement is predicted using IS: 8009 (part-1)-1976.

A typical calculation for estimating the bearing capacity of foundation is given in annexure-IV

7 FOUNDATION RECOMMENDATIONS

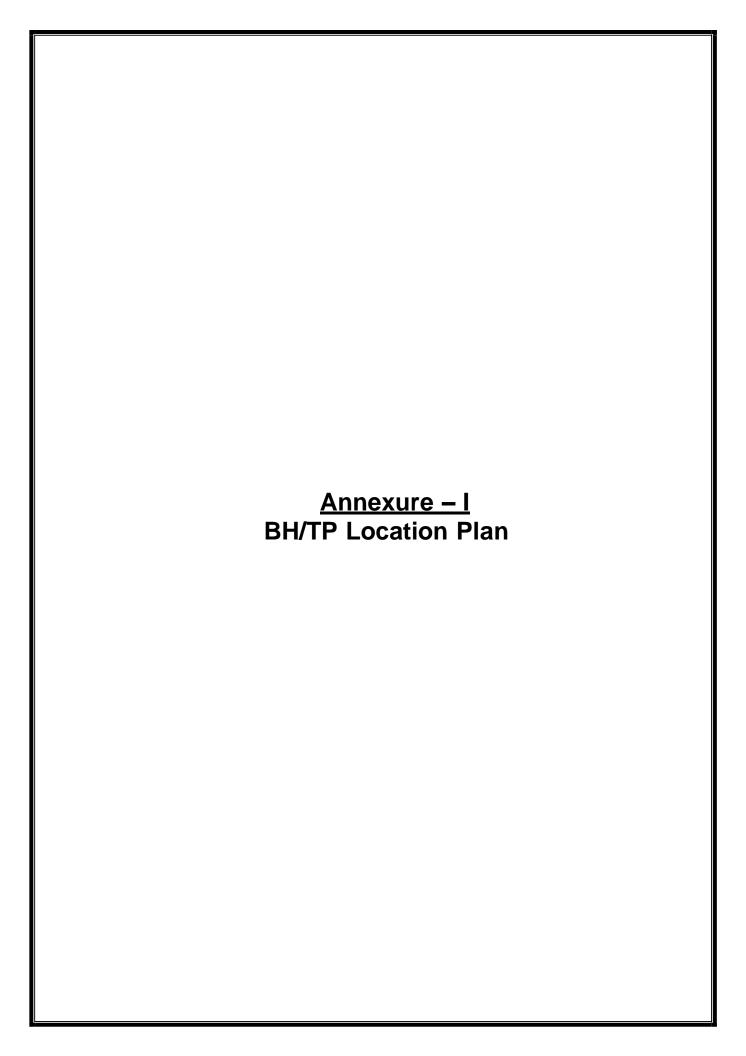
Based on the field and laboratory tests the following recommendations are given

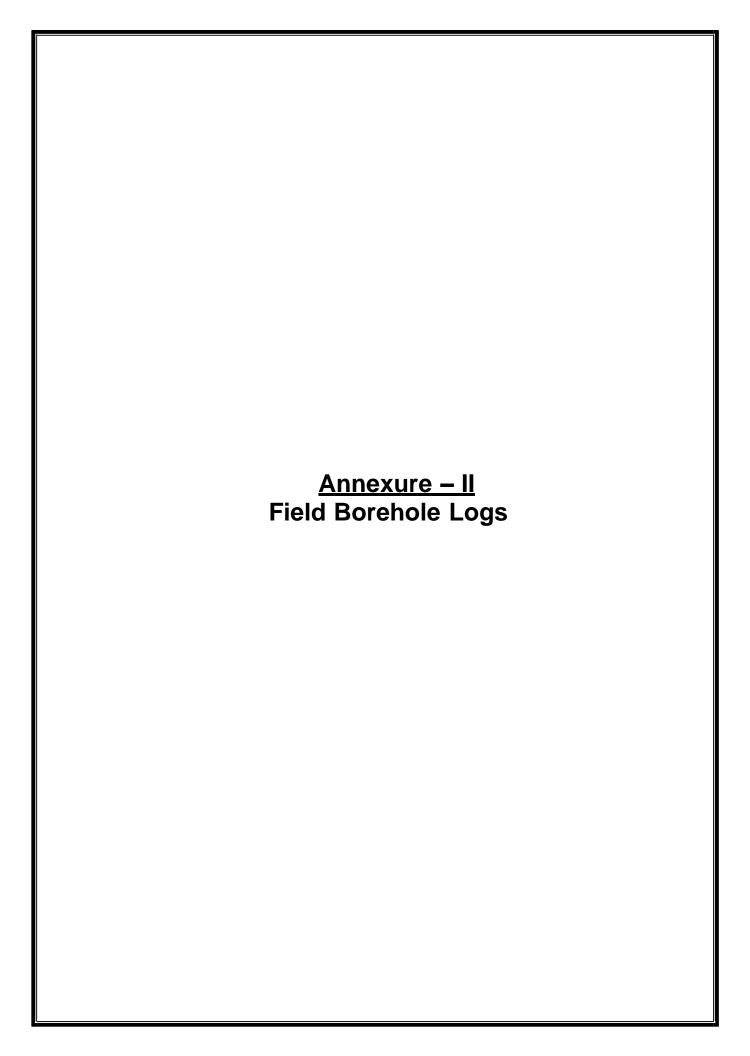
- Open foundations are recommended in the form of isolated footing
- ♣ The recommended safe bearing capacity is 5t/m². This is based on the isolated footing of width 2.0m at 2.0m depth below the natural ground level. The actual size would be based on the structural loads.
- The recommended depth of foundations should at 2.0m below natural ground level.

- ♣ All foundation works should be done in dry condition only and foundation pits should be filled with well compacted morum
- ♣ Foundation should rest on 100mm thick 1:4:8 concrete mat

M. Chandrasekhar Babu

Director







Siri GeoSolutions Pvt Ltd

Civil & Geotechnical Engineering Services

Project : Geotechnical Investigation Report for Storm Water Drainage Scheme of Muzaffarpur City

BOREHOLE RECORD

Boring Method : Auger Drilling Bore Hole No. : BH-1

Boring Diameter : 100 mm R.L : N/A

Depth of GWT : 6.0m Location : Ref Annexure-1

Structure : - Date Commenced :: 19.08.2014

Boring Equipment : Screw Type Auger Date Completed :19 08 2014

Boring	Equipment	: Screw	Type	10000			Date Co	mpleted :19.08.2014			
Depth (m)	Sample / In-sit			'N	SPT ' Value		Depth (BGL)	Description	RL (m)	Log	Remarks / Other
(111)	Depth (m)	Type	15	30	45	N	(m)		(111)		Tests
1.00	1.50 - 1.95	SPT	1	1	1	2					
3.00	3.00 - 3.45	UDS									
5.00	4.50 - 4.95	SPT	1	1	2	3		Greyish yellow clayey silt(CL)			
6.00	6.00 - 6.45	SPT	2	3	2	5		sam(22)			
7.00 8.00	7.50 - 7.95	SPT	2	3	4	7					
9.00	9.00 - 9.45 10.00 - 10.45	UDS SPT	1	4	4	8	10.00		-10.00		
11.00	10.00 10.40	Ol 1			-	U	10.00	Borehole Terminated	10.00		
12.00											
13.00											
14.00											
16.00											

SPT : Standard Penetration Test CR : Core Recovery

UDS : Undistrubed Sample RQD : Rock Quality Designation

GWT : Ground Water Table RP: Rate of Penetration

Client: M/s. Darashaw and Company Private Limited, Hyderabad Geotechnical Consultant: M/s. Siri GeoSolutions Pvt Lid., Hyderabad.



Siri GeoSolutions Private Limited Civil & Geotechnical Engineering Services

Trial Pit- Data Sheet

Project

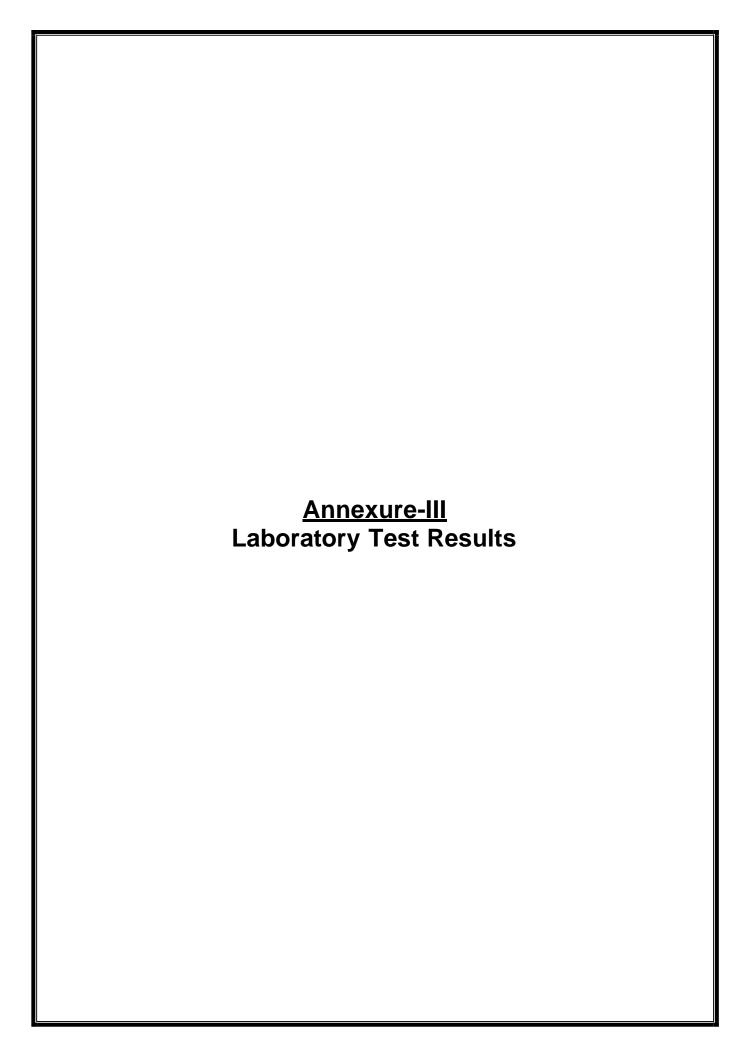
Geotechnical Investigation Report for Storm Water Drainage Scheme for Muzaffarpur Municipal Corporation.

Location- Trial Pit -TP-01

Depth	Description	Sa	mple	Section	Log	Remarks
(m)	of strata	Type	Depth	Section	Log	Hemaiks
1.00	Greyish yellow clayey silt	DS	1.00			
2.00		DS	2.00			
		UDS	2.50			
3.00				Trila Pit-TP01		

Location- Trial Pit -TP-02

Depth	Description	Sa	mple	Section	Log	Remarks
(m)	of strata	Type	Depth	Section	Log	Hemarks
1.00	Greyish yellow clayey silt	DS	1.00			
2.00		DS	2.00			
		DS	2.50			
3.00				Trila Pit-TP02		



Proj	ect:	Geotechn	ical Inv	vestigation Re	port fo	r Storm	Water	Draina	ge Schem	ne of Muz	affarpur Ci	ty							
								Labo	oratory Te	est Resul	ts								
		Туре			IS Classification		Atterb Lim	erg's nits		Graii	n Size Distr	ibution (%	b)			Dens (kN/m3		Shea	ar Test
Borehole	Depth (m)	of sample	SPT "N"	Soil Description	IS ssific	NMC (%)	WL	WP	Gravel		Sand		Silt &	Clay	Specific Gravity	Bulk	Dry	C (kPa)	φ'
					Clas		***	"	Graver	Coarse	Medium	Fine	М	С			•		
	1.50	SPT	2						0	0	0	2	9	8					
	3.00	UDS	-			19.5	Non F	Plastic	0	0	1	2	68	29	2.66	17.90	14.98	16.30	8.00
	4.50	SPT	3	Greyish					0	0	2	7	9	7					
BH-01	6.00	SPT	5	yellow	CL	17.4	Non F	Plastic	0	0	0	5	8	9					
	7.50	SPT	7	clayey silt					0	0	1	8	9	1					
	9.00	UDS	-			18.6	Non F	Plastic	0	0	4	10	65	18	2.67	18.50	15.60	32.60	12.00
	10.00	SPT	8						0	0	7	12	8	4					
	1.00	DS		Greyish					0	0	2	6	9	2					
TP-1	2.00	DS		yellow	CI				0	0	1	5	9	4					
	2.50	UDS		clayey silt		15.0			0	0	4	5	69	22		18.20	15.83	14.90	6.30
	1.00	DS		Greyish					0	0	0	12	8	8					
TP-02	2.00	DS		yellow	CI	11.6			0	0	0	9	9	1					
	2.50	DS		clayey silt					0	0		7	9	2					



Siri GeoSolutions Pvt Ltd Civil & Geotechnical Engineering Services

Project:

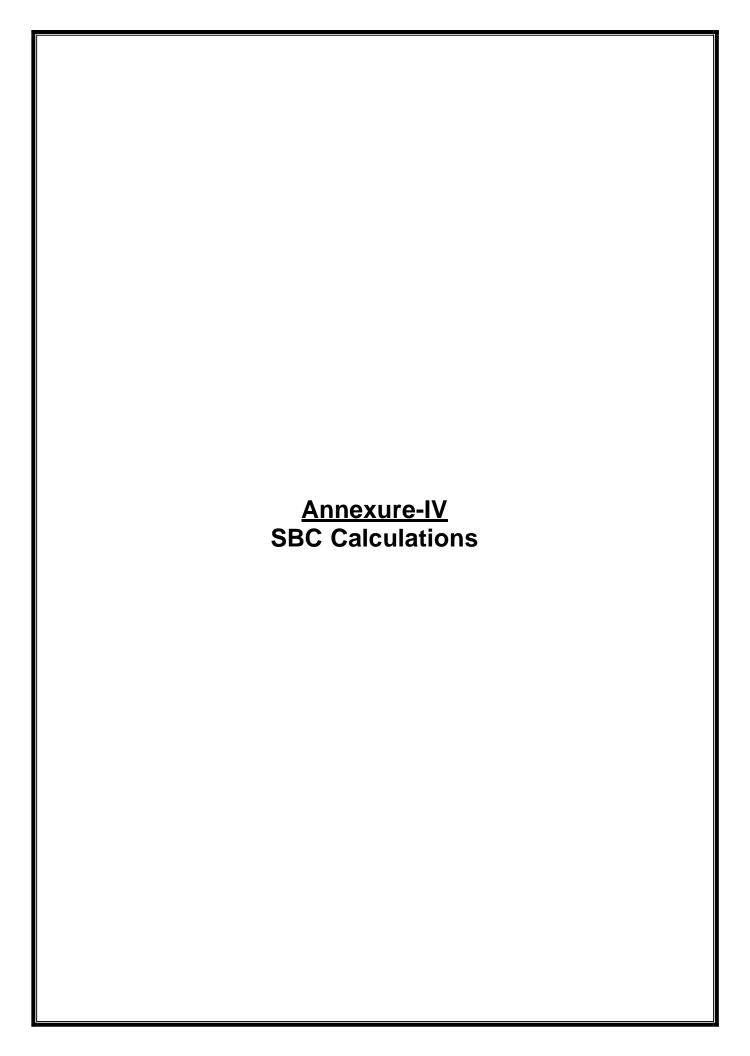
Geotechnical Investigation Report for Storm Water Drainage Scheme of Muzaffarpur City

Soil - Chemical Analysis - Results

				· · · · · · · · · · · · · · · · · · ·		
		S	oil-Chemical Ana	alysis-Results		
				PH	Chloride	Sulphate
Location	Depth (m)	Color	Odor	(10% suspension)	content %	as SO ₃ (%)
			Characteristic			
TP-1	2	Grayish	Soil odor	7.62	0.17	0.22

Water - Chemical Analysis - Results

	Water-C	Chemical Analysis-Results	
Location	PH (10% suspension)	Chloride Content mg/l	Sulphate as SO ₃ (%)
BH-1	7.63	79	213



SIRI GEOSOLUTIONS PVT LTD

Civil & Geotechnical Engineering Services

Bearing Capacity Based on Shear Criteria (Based on IS: 6403-1981)

Bore hole number	=	BH-1	
Ground water table location from FGL/NGL	=	0 m	
Unit weight of soil (γ)	=	1.8 t/m2	
Submerged unit of soil (γ')	=	0.8 t/m2	
Type of footing :	=	Square footing	
Width of footing (B)	=	2 m	
Length of footing(L)	=	2 m	
Depth of footing from NGL (D _f)	=	2 m	
Average N value at base of footing	=		
Angle of internal friction at that depth (ϕ)	=	8 deg.	(Lab results)
Cohesion (C)	=	1.6 t/m2	
If f value is in between local & general shear failure IS: 6403 - 1981 the SBC value is interpolate between	•		
f value for General shear failure	=	36 deg	
φ value for Local shear failure	=	29 deg.	
Local shear failure :			
$\varphi' = \tan^{-1} ((2/3) \tan \varphi)$	=	5 deg.	
Cohesion (C) C' Bearing capacity factors (IS: 6403-1981, table 1):	=	1.07 t/m2	
Nc'	=	6.49	
Nq'	=	1.57	
$N\gamma'$	=	0.45	
Shape factors for square footing (IS:6403-1981,table	le 1):		
Sc	=	1.3	
S_q	=	1.2	
Sγ	=	0.8	

SIRI GEOSOLUTIONS PVT LTD

Civil & Geotechnical Engineering Services

Bearing Capacity Based on Shear Criteria (Based on IS: 6403-1981)

Inclination factors (IS:6403-1981)

$$i_c = i_q = i\gamma = 1$$

Depth factors(IS: 6403-1981,cl:5,1.2.2)

$$d_c = 1 \! + \! 0.2 \ ^*(D_f\!/b) \ (N_\phi \,)^{1/2}$$

$$d_{q} = d_{y} = 1 \text{ for } \phi < 10^{0}$$

$$d_q = d_v = 1+0.1 * (D_f/B)(N_{\phi})^{1/2} \text{ for } \phi > 10^0$$

Where,

 $N\varphi = tan^2(45+\varphi/2)$ = 1.19

Therefore,

c = 1.22

 d_q = 1

 $d_{v} = 1$

Over burden pressure $(q = \gamma' * D_f)$ = 1.6 t/m2

Factor of safety (F.S) = 2.5

Based on shear failure criteria Ultimate bearing capacity (UBC)

for local shear failure as per IS:6403 - 1981:

$$UBC = 2/3*C' *N_c' *S_c *d_c *i_c + q*(N_q' - 1)*S_q *d_q *i_q + 0.5 *B*\gamma *N_{\gamma'} *S_{\gamma} *d_{\gamma} *i_{\gamma} *w$$

= 17.88 t/m2

Safe bearing capacity as per shear criteria,

$$(SBC)local = UBC/F.S = 7.15 t/m2$$

Recommended SBC = 5 t/m^2

SIRI GEOSOLUTIONS PVT LTD Civil & Geotechnical Engineering Services

BEARING CAPACITY BASED ON SETTLEMENT CRITERIA

(Based on IS: 8009 (Part-1)-1976)

Type of foundation = Square footing

Unit weight of soil (γ) = 1.8 t/m³

Width of foundation (B) = 2 m

Length of foundation (L) = 2 m

Depth of foundation from NGL (D_f) = 2.0 m

Bearing pressure at founding level (q) = 5 t/m²

Depth of pressure bulb (1.50*B) H = 3.0 m

Bearing capacity Based on Settlement

Immediate settlement = $H^*\Delta p/E_s$

where E_s = Elastic modulus of sand

H = Depth of sand layer

 Δp = Incrimental pressure at centre of layer

Type of soil = Silty clay / clayey silt

Depth of layer (H) = 3.0 m

Increase in pressure $\Delta p = q * B * L / (B+z)*((L+z)$

 $\Delta p = 1.63 \text{ t/m}^2$

 E_s = 250 t/m² (based on shultz &

& Muhs curves)

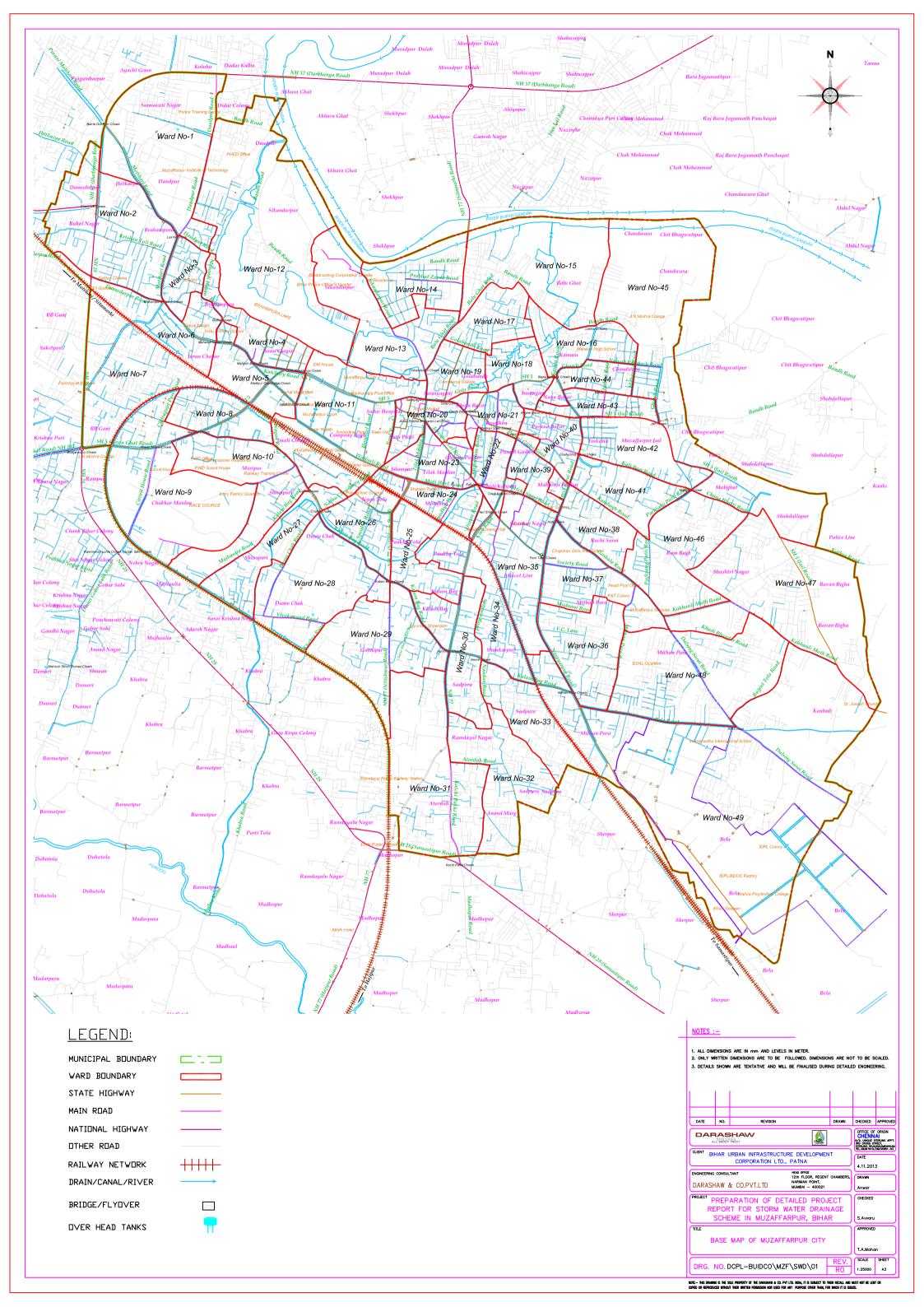
settlement of layer = 19.6 mm

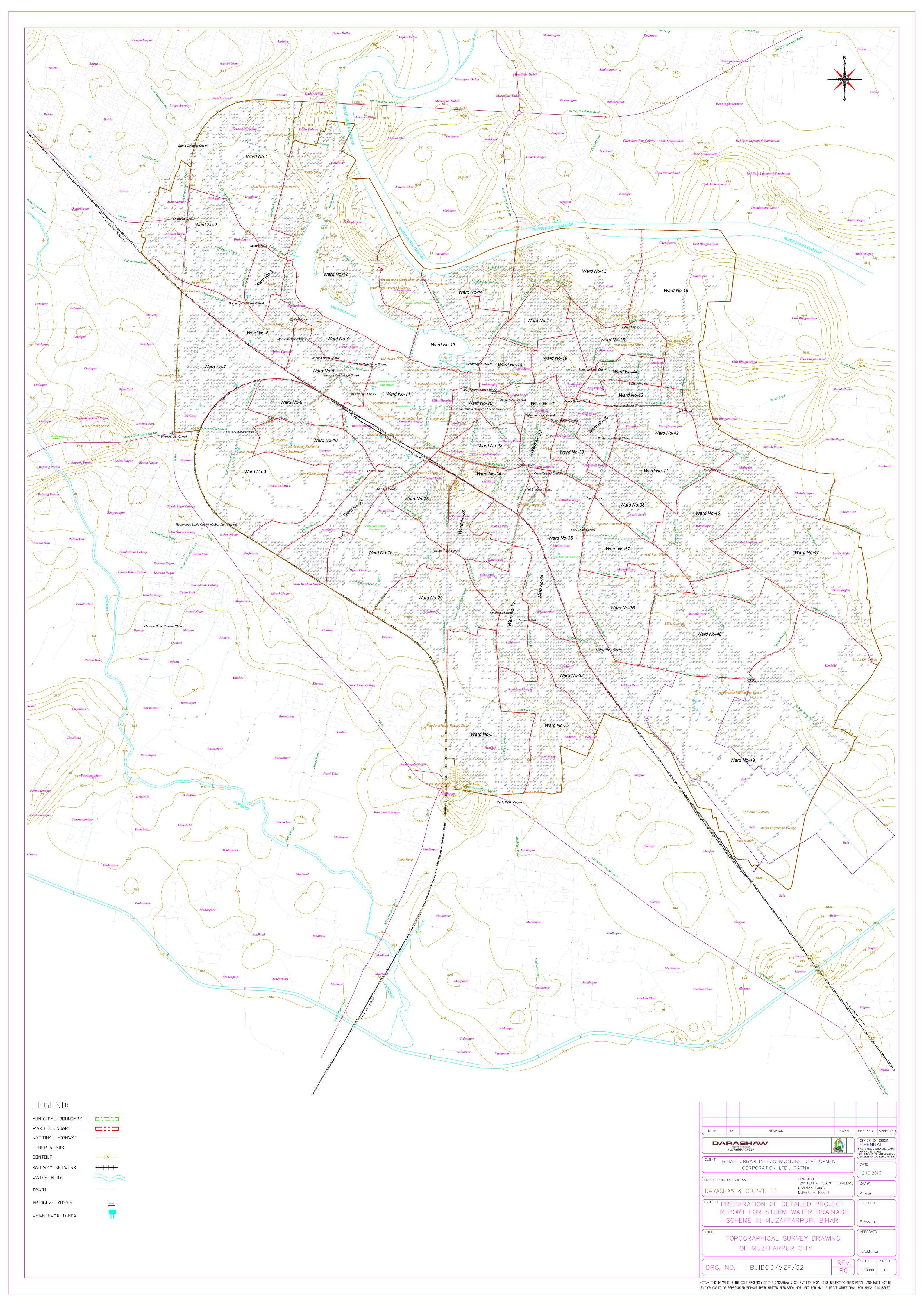
Total settlement = 19.6 mm

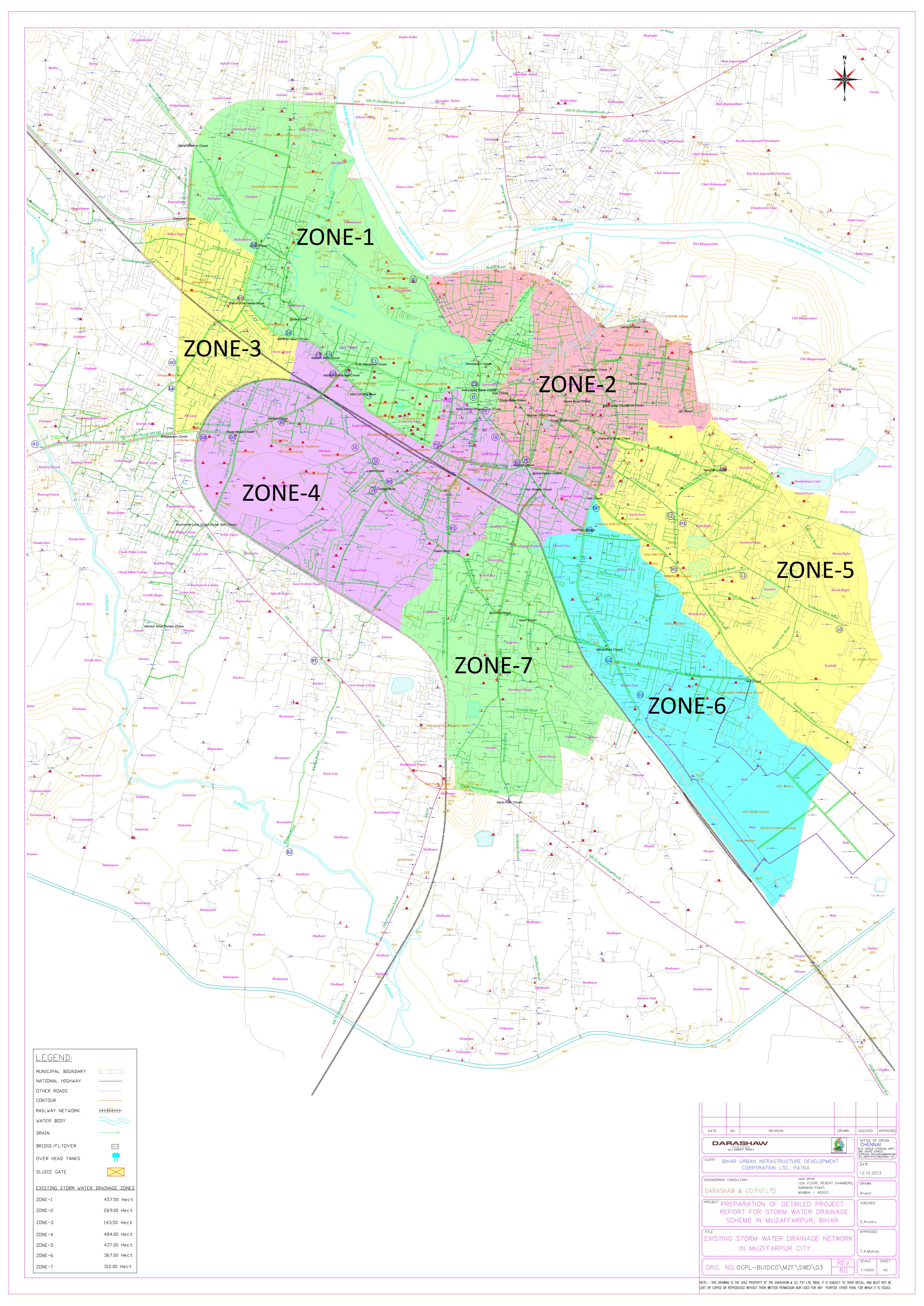
Depth correction factor = 1.00 (as per Fig.12 of IS:8009-pt1)

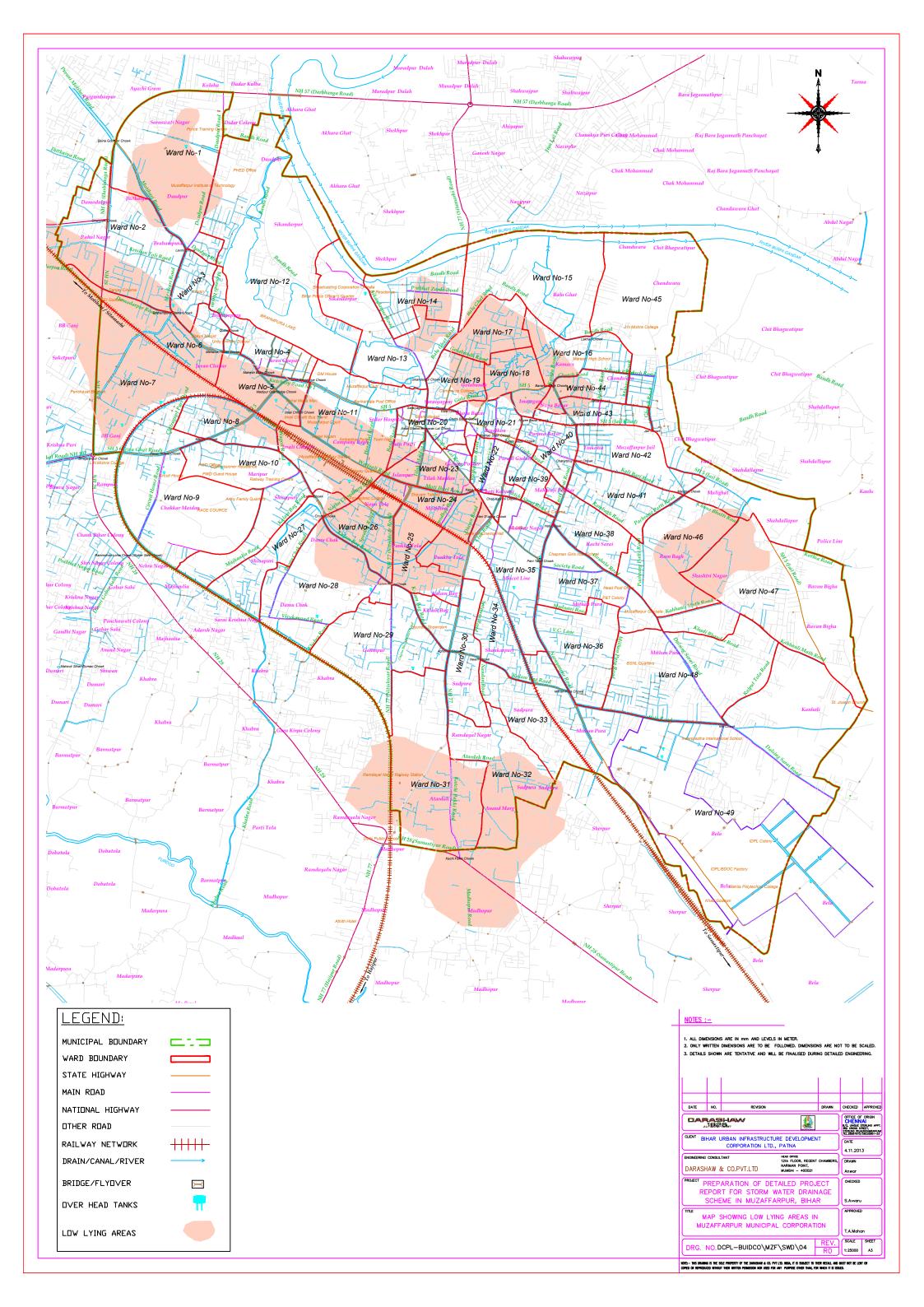
Rigidity factor = 0.8 (as per clause 9.5.2 of IS 8009-pt1)

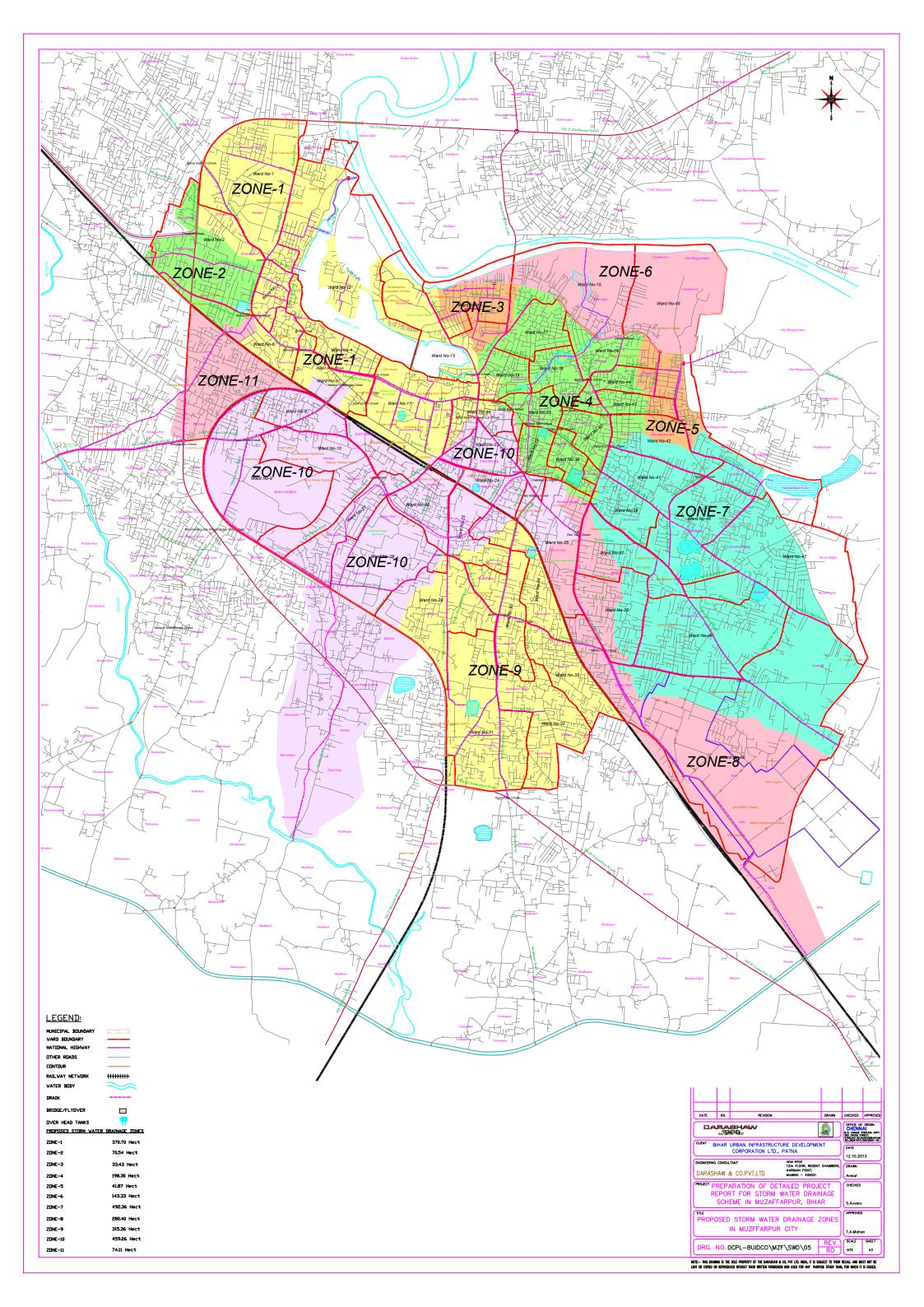
Corrected settlement = 16 mm

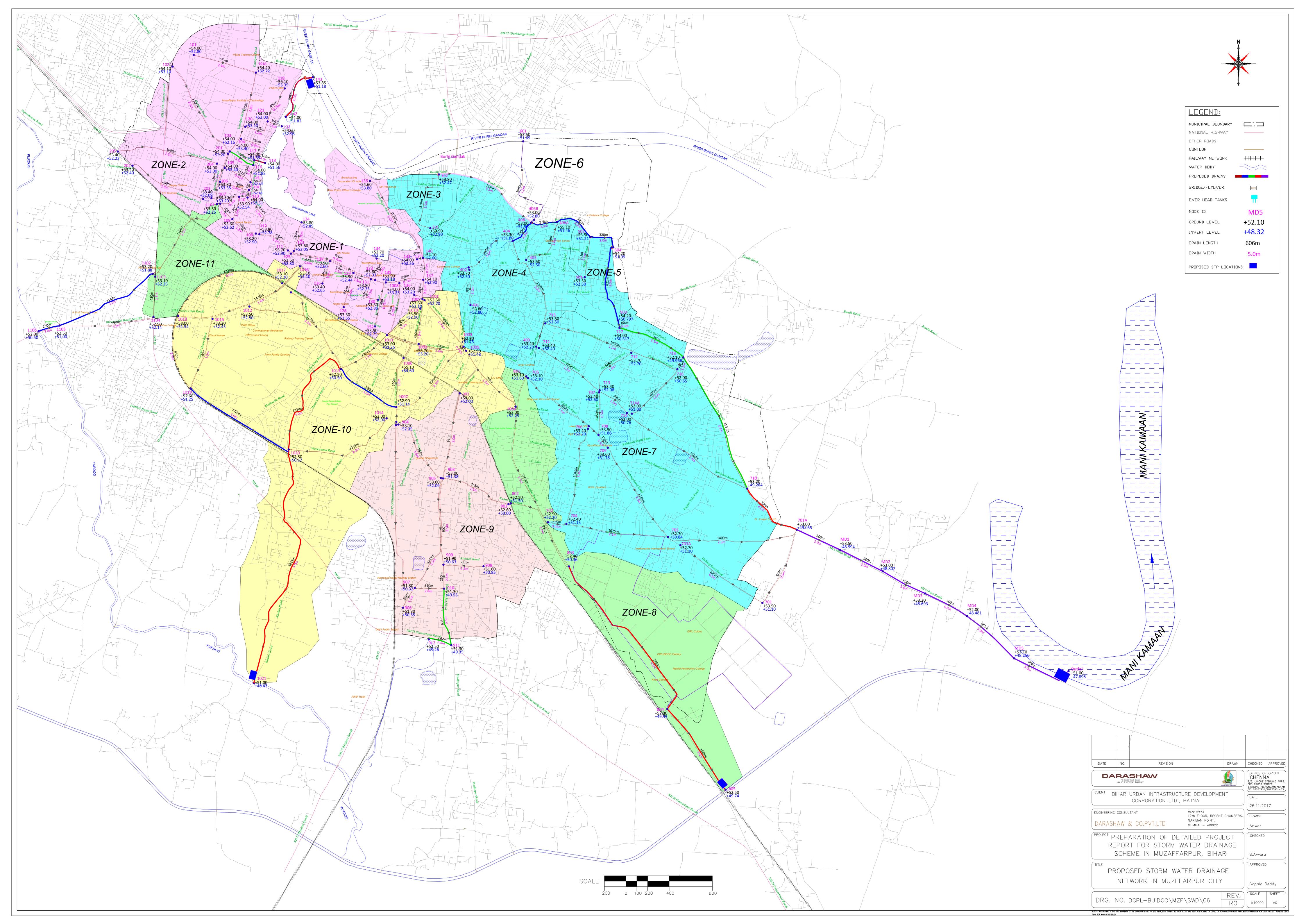


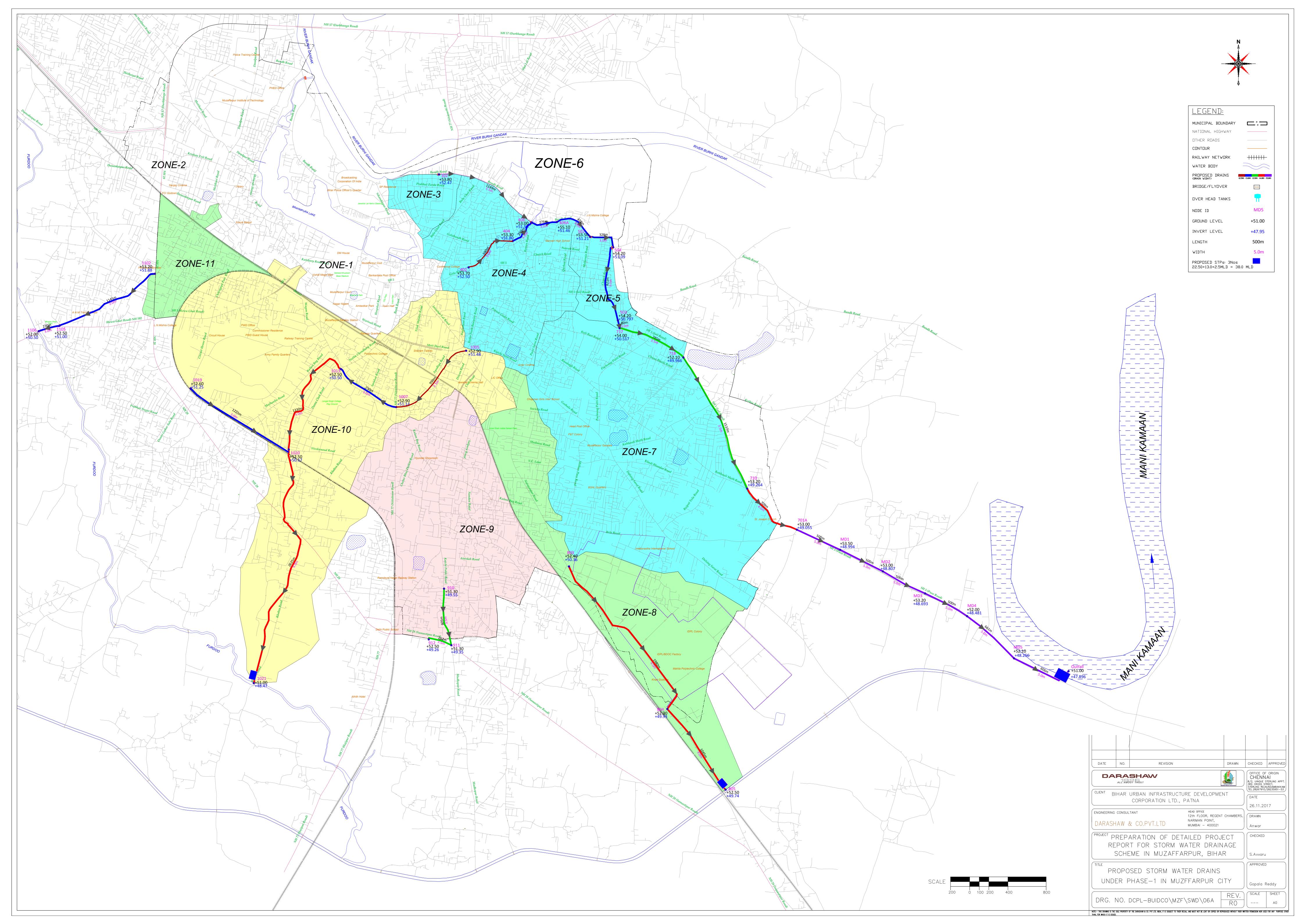


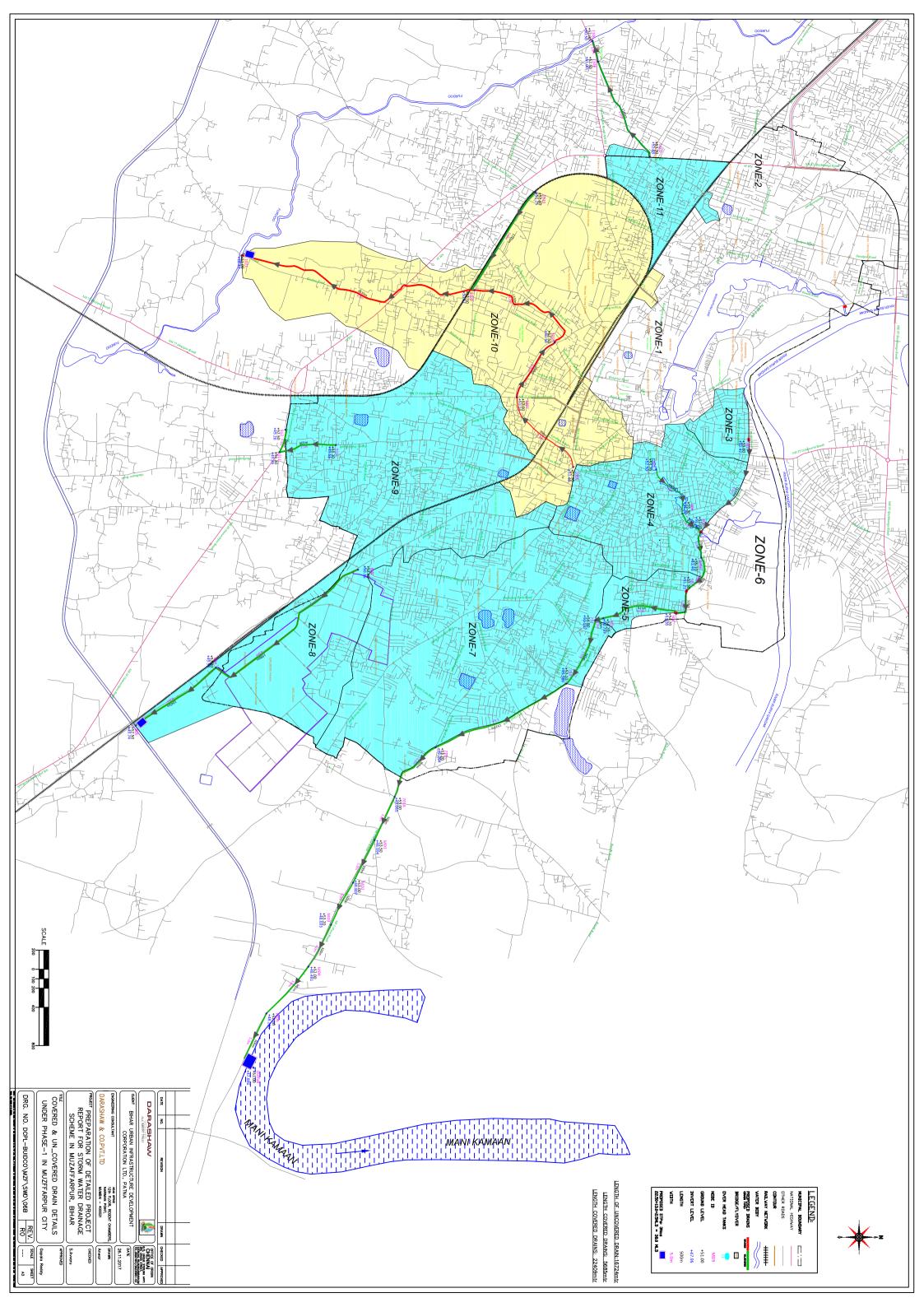


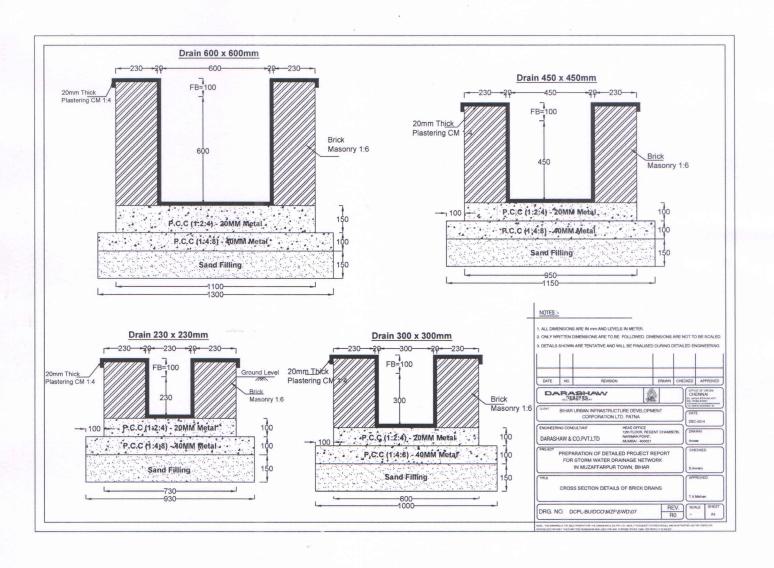


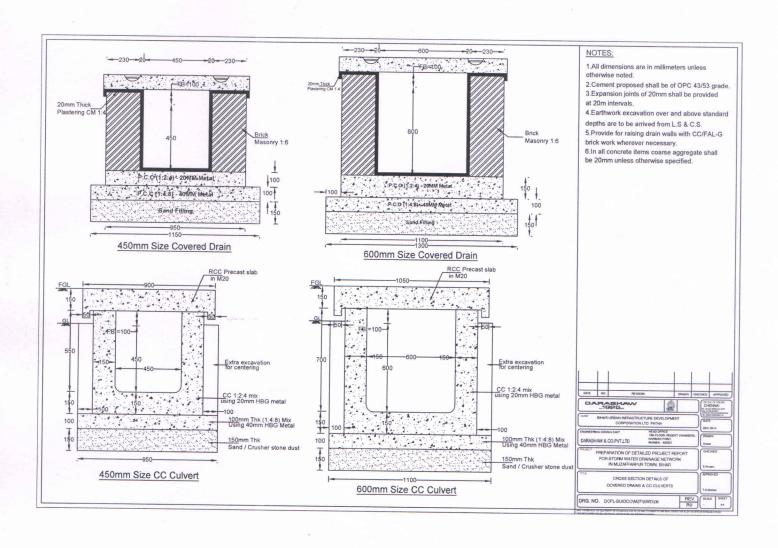


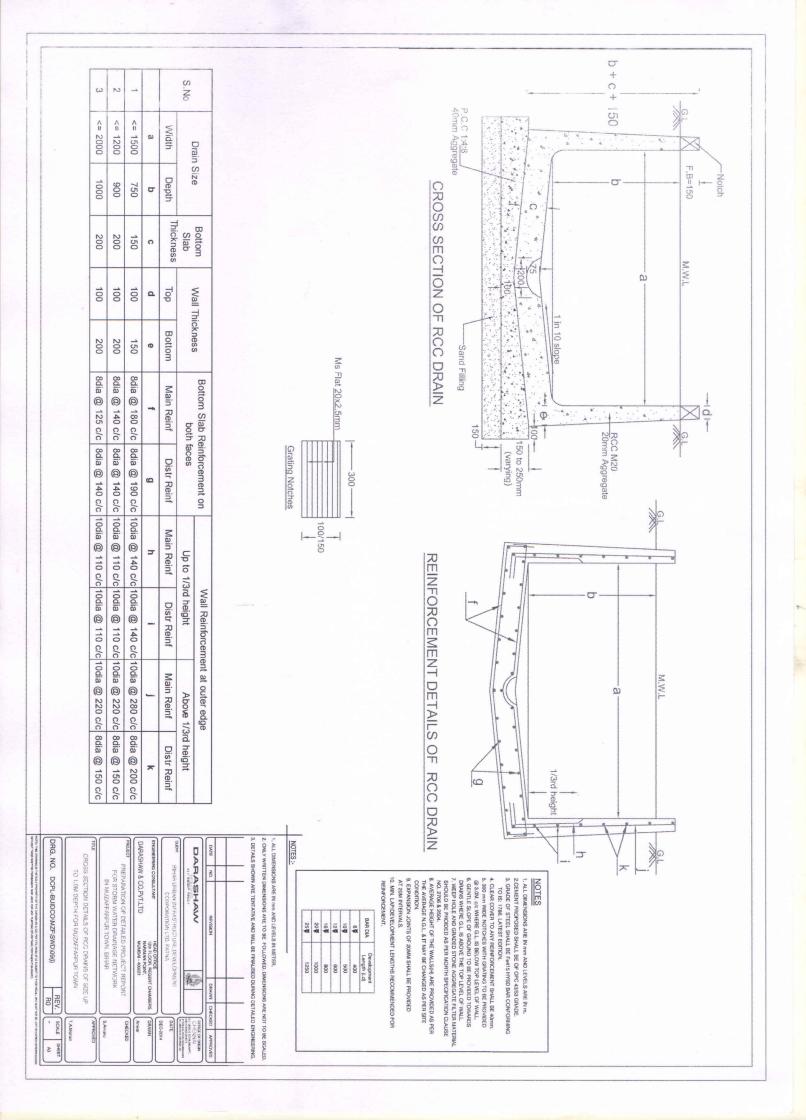


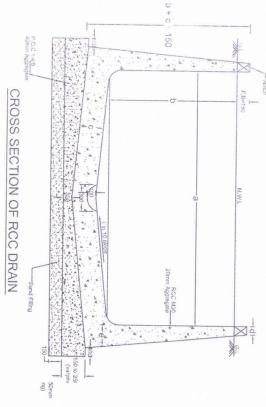


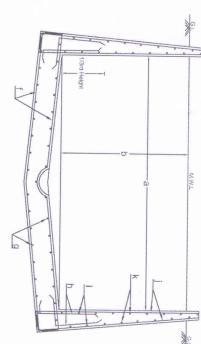












4. CLEAR COVER TO ANY REINFORCEMENT SHALL BE 40mm,
5. 300 mm WIDE NOTICHES WITH GRATING TO BE PROVIDED
(@ 30M, co) WHERE CLL IS BELOW TOP LEYEL, DE WALL
16. GENTLE SLOPE OF GROUND TO BE PROVIDED TOWARDS
DAWNS WHERE CLL IS ABOVE THE TOP LEYEL OF WALL
7. WEED HOLE AND GRADED IS TOWER AGGREGATE FUTER MATERIAL
8HOULD BE PRODUED AS PER MORTH SPECIFICATION CLAUSE
NO 2704 5 2404.

1. ALL DIMENSIONS ARE IN mm AND LEVELS ARE IN m.
Z.CEMENT PROPOSED SHALL BE OF OPG 43/93 GRADE.
8. GRADE OF STEEL SHALL BE FA1'S HYSD BAR CONFORMING
TO IS: 1786, LATEST EDTION.

NO. 2706 & 2504.

8. AVERAGE HEIGHT OF THE WALLS(H), ARE PROVIDED AS PER THE AVERAGE N.G.L. & IT MAY BE CHANGED AS PER SITE CONDITION.

9. EXPANSION JOINTS OF 20MM SHALL BE PROVIDED AT 20M INTERVALS.

10. MIN. LAP/DEVELOPMENT LENGTHS RECOMMENDED FOR

REINFORCEMENT.

REINFORCEMENT DETAILS OF RCC DRAIN

500 600 800 1000 1250

	//s Flat <u>20x2,5mm</u>
Grating Notches	100/150

S.No	Dra	Drain Size	Bottom Slab Thick ness	Wall	Wall Thickness	Bottom Slab Reini Fa	Bottom Slab Reinforcement on both Faces		Wall Reinforcement on both Faces	17.
	Width	Depth		Тор	Bottom	Main Reinf	Distr Reinf	Main Boinf	Distr Daint	
	,	-		***************************************		HILLSNI HISKA	נונים ווכוווו	IVIdili Kelli	DIST KEINT	
	3	٥	C	a	0	-	70	_		
j	<=2500	1100-1250	250	100	200	10dia @ 140 c/c	8dia @ 140 r/c	10dia @ 105 c/c	241 @ cibs	
J	1-2005	1000 1500	200	400			The state of the s	70000 Ex 10000	OCIG ENTLY	TOUR @ 220 C/C 8019 @ 770 C/C
3 6	2000	OOCT -OOCT	250	IOU	250	10dia @ 125 c/c	8dia @ 110 c/c	10dia @ 100 c/c	8dia @ 100 c/c	10dia @ 200 c/c 8dia @ 175 c/c
U	S=3500	OC/T	350	100	300	10dia @ 110 c/c	8dia @ 140 c/c 10dia @ 125 c/c	10dia @ 125 c/c	8dia @ 125 c/c	12dia @ 250 c/c 10dia @ 200 c/c
4	<=4000	2000	400	100	350	12dia @ 125 c/c	10dia @ 125 c/c	12dia @ 125 c/c 10dia @ 125 c/c 12dia @ 110 c/c 10dia @ 110 c/c	10dia @ 110 c/r	
(J)	<=5000	2500	400	100	400	12dia @ 125 c/c	10dia @ 125 c/c	12dia @ 125 c/c 10dia @ 125 c/c 12dia @ 110 c/c 10dia @ 110 c/c 12dia @ 125 c/	10dia @ 110 c/c	1 200 - 1 0 1 0 1 0 0 0 0 0 0 0 0 0 0 0 0 0
n	117775	3770	200	400	N Pro Pro			The Carte of C	110 C 22001	-
1 0	OD00-	05/7	400	100	450	12dia @ 125 c/c	10dia @ 125 c/c	10dia @ 125 c/c 12dia @ 110 c/c 10dia @ 110 c/c	10dia @ 110 c/o	12dia @ 220 c/c 10dia @ 200 c/c
/	<=6000	3000	400	100	450	12dia @ 125 c/c	10dia @ 125 c/c	12dia @ 125 c/c 10dia @ 125 c/c 12dia @ 110 c/c 10dia @ 110 c/c	10dia @ 110 c/c	12dia @ 220 c/c 10dia @ 200 c/c

TES:

, ALL DIMENSIONS ARE IN mm AND LEVELS IN METER.

2. ONLY WRITTEN DIMENSIONS ARE TO BE FOLLOWED, DIMENSIONS ARE NOT TO BE SCALED.
3. DETAILS SHOWN ARE TENTATIVE AND WILL BE FRALISED DURING DETAILED ENGINEERING.

DATE NO. REPISCON DAWN O-ECORED APPROVED.

DARASHAW

ALT TRADET FINEL

COSTO-DANIAN LIFE DE NELOPERNY

COSTO-DANIAN LIFE DE NELOPERNY

COSTO-DANIAN LIFE DE NELOPERNY

COSTO-DANIAN LIFE DE NELOPERNY

DECORI

MARIEN ACCORD

MARIEN ACCORD

ANDREASH OF DET MILED PROUECT REPORT

PORCOED

ORGODO

ORGOD

OF SIZE ABOVE I DM DETALS OF FICE DRAINS

OF SIZE ABOVE I DM DEPTH FOR MUZAFFABPUR TOWN

DRO, NO. DCPL-BUIDCO/MZP\SWD\09(ii)

RO

RO

SOME
SHEET

APPROVED

APPROVED

APPROVED

TAMPING

SOME
SHEET

BOWLE

SOME
SHEET

APPROVED

TAMPING

BOWLE
SHEET

APPROVED

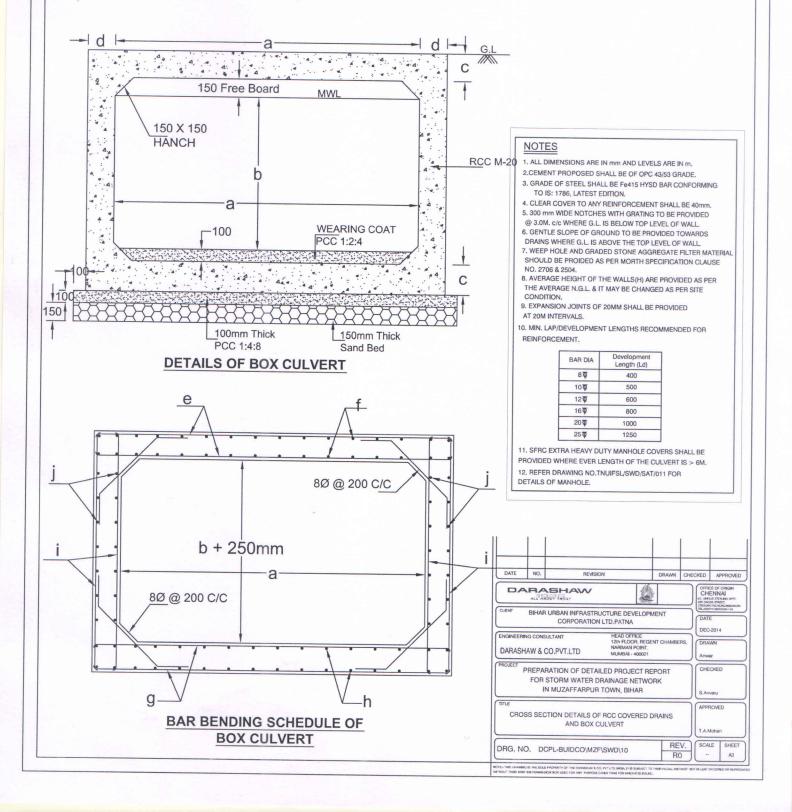
TAMPING

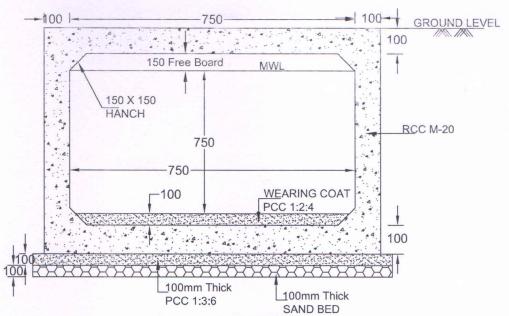
TOWN

TOW

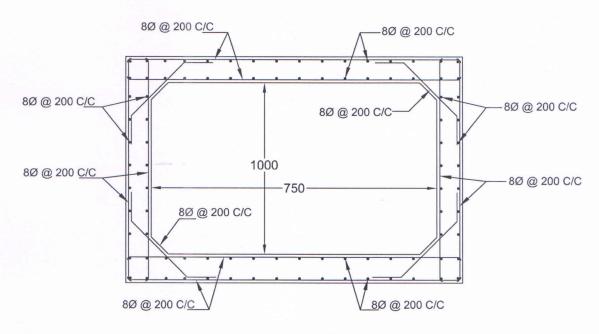
FOR STORM WATER DRAINAGE NETWORK IN MUZAFFARFUR TOWN, BIHAR

S.No	Size of Cuivert		Thickness of	Thickness of	Top Slab		Bottom slab		Walls	
	Width	Depth	top slab and bottom slab	walls	Main Reinf	Distr Reinf	Main Reinf	Distr Reinf	Main Reinf	Distr Reinf
	а	b	С	d	е	f	g	h		-
1	750	750	100	100	10dia @ 200 c/c	8dia @ 175 c/c	10dia @ 200 c/c		10dia @ 200 c/c	8dia @ 175 c/c
2	900-1000	<=1000	150	150	10dia @ 175 c/c		10dia @ 175 c/c		10dia @ 175 c/c	8dia @ 175 c/c
3	1200	<=1000	200	200	10dia @ 150 c/c		10dia @ 150 c/c			8dia @ 1/3 c/c
4	1500	<=1000	150	150	10dia @ 125 c/c	8dia @ 175 c/c	10dia @ 100 c/c			8dia @ 175 c/c
5	1800	<=1200	200	200	12dia @ 150 c/c	8dia @ 140 c/c	12dia @ 150 c/c			8dia @ 140 c/c
6	2000	<=2000	200	200	12dia @ 120 c/c	8dia @ 140 c/c	12dia @ 120 c/c			
7	2500	<=1250	300	250	12dia @ 100 c/c				12dia @ 120 c/c	
8	3000	<=1500	350	300	16dia @ 150 c/c	10dia @ 125 c/c	16dia @ 150 c/c	10dia @ 125 c/c	16dia @ 150 c/c	10dia @ 1/3 c/
9	3500	<=1750	400	350	16dia @ 140 c/c	10dia @ 110 c/c	16dia @ 125 c/c	10dia @ 110 c/c	16dia @ 125 c/c	10dia @ 140 c/
10	4000	<=2000	450	400	16dia @ 120 c/c	12dia @ 140 c/c	16dia @ 100 c/c	12dia @ 140 c/c	16dia @ 100 c/c	12dia @ 150 c/c
11	5000	<=2500	450	400	16dia @ 120 c/c	12dia @ 140 c/c	16dia @ 100 c/c	12dia @ 140 c/c	16dia @ 100 c/c	12dia @ 150 c/c
12	5500	<=2750	450	450	16dia @ 120 c/c	12dia @ 140 c/c	16dia @ 100 c/c	12dia @ 140 c/c	16dia @ 100 c/c	12dia @ 150 c/c
13	6000	<=3000	500	450	16dia @ 120 c/c	12dia @ 140 c/c	16dia @ 100 c/c	12dia @ 140 c/c	16dia @ 100 c/c	12dia @ 150 c/c





DETAILS OF COVERED DRAIN



BAR BENDING SCHEDULE OF COVERED DRAIN

NOTES

- 1. ALL DIMENSIONS ARE IN mm AND LEVELS ARE IN m. 2.CEMENT PROPOSED SHALL BE OF OPC 43/53 GRADE.
- 3. GRADE OF STEEL SHALL BE Fe415 HYSD BAR CONFORMING TO IS: 1786, LATEST EDITION.
- 4. CLEAR COVER TO ANY REINFORCEMENT SHALL BE 40mm. 5. 300 mm WIDE NOTCHES WITH GRATING TO BE PROVIDED
- @ 3.0M. c/c WHERE G.L. IS BELOW TOP LEVEL OF WALL.
 6. GENTLE SLOPE OF GROUND TO BE PROVIDED TOWARDS
- DRAINS WHERE G.L. IS ABOVE THE TOP LEVEL OF WALL.
 7. WEEP HOLE AND GRADED STONE AGGREGATE FILTER MATERIAL
 SHOULD BE PROIDED AS PER MORTH SPECIFICATION CLAUSE
 NO. 2706 & 2504.
- AVERAGE HEIGHT OF THE WALLS(H) ARE PROVIDED AS PER THE AVERAGE N.G.L. & IT MAY BE CHANGED AS PER SITE CONDITION.
- 9. EXPANSION JOINTS OF 20MM SHALL BE PROVIDED AT 20M INTERVALS.

10. MIN. LAP/DEVELOPMENT LENGTHS RECOMMENDED FOR REINFORCEMENT.

BAR DIA	Development Length (Ld)		
8 👨	400		
10Ф	500		
12₹	600		
16♥	800		
20₹	1000		
250	1250		

- 11. SFRC EXTRA HEAVY DUTY MANHOLE COVERS SHALL BE PROVIDED AT EVERY 10M INTERVAL.
- 12. REFER DRAWING NO.TNUIFSL/SWD/SAT/011 FOR DETAILS OF MANHOLE.

DATE	NO.	REVISION		DRAWN	CHECKED	APPROVED	
CUENT BIHAR UBBAN INFRASTRUCTURE DEVELOPMENT CORPORATION LTD. PATNA						OFFICE OF ORIGIN CHENNAI a2, UNIQUE STEPLING APPT. 3ND ORIOSS STEELS. STEPLING RD NUMBERSHOOM TEL 284781C0823961-63	
	ING CONSULTAR	12th f NARII	O OFFICE FLOOR, REGEN MAN POINT, BAI - 400021	T CHAMBE			
PROJECT	FOR STO	ION OF DETAILED PR PRM WATER DRAINAG MUZAFFARPUR TOWN	E NETWOR	110	CHE S.Av	CKED	
mile (CROSS SEC	TION DETAILS OF CO	VERED DRA	AINS		ROVED	
200 1	IO DCBI	BUIDCO\MZF\SWD\	4.4	RE	V. SCA	LE SHEET	

